

KWANONKUTHULA MAIN OUTFALL SEWER & PUMPING MAIN UPGRADE PLETTENBERG BAY

19118 CGAC
ADDITIONAL TESTING

SOIL INVESTIGATION



APRIL 2025

Author:

Llewelyn Heathcote



OUTENIQUA LAB (Pty) Ltd

Registration No. 95/07742/07

Materials Testing Laboratory

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R-LET-1-3 Apr-08



T0347

28 May 2025

Neil Lyners
GEORGE

Attn: F van Eck

CONTRACT NO 19118CGAC : KWANOKUTHULA MAIN OUTFALL SEWER & PUMPING MAIN

Term of reference

Outeniqua Lab (Pty) Ltd was requested by Neil Lyners on behalf of the Bitou Municipality to excavate 27 trial pits to a maximum depth of 2.5m or refusal along the proposed pipeline route and comment on the findings.

Scope of work

The site investigation included a series of trial pits labelled TP1 to TP27, as indicated on the layout plan, as well as insitu DCP penetration tests at each trial pit, excluding TP20 due to coarse aggregate, with laboratory testing carried out on soil samples taken.

Results from the site investigation

The site is located on the southern side of the N2 near the Kwanokuthula intersection, moving along though the settlement in a south westerly direction and then in an east to west direction on the southern boundary along the edge break of the settlement. The site also included a section on the southern side of the N2 from the Kwanokuthula intersection to the Schoongezigt development.

The topography of the site is generally flat with gently sloped plains to very steep slopes on the southern boundary of the settlement. All areas along the pipeline were accessible with a TLB, however there are areas that could require a tracked excavator.

The underlying geology of the area consists of Table Mountain series weathered quartz sandstone of the Peninsula formation as well as some Enon conglomerates, sandstones, silt stones and clays and soil derived from the weathering of these rocks. This formation is typically deeply weathered in the area, forming fine gravels to silt/silty clays soil profiles, as well as rock outcrops on surface near the settlement boundary.

The test pits conducted indicate that the soil profiles are dominated by silty sands, Ferricrete, silt/silty clays and coarse gravels, this correlates with the local geology of the area.

The silty sands with and without Ferricrete and silt/silty clays could not be classified according to TRH14, indicating that the material is less than a G10 quality, with only the well developed Ferricrete and the Sandy Gravel with Boulders providing a TRH14 classification of G6 quality.

The silt/silty clay as well as the Sandy Clay samples range between ML (silt) and MH (elastic silt) classification with low potential expansiveness.

None of the silty sands could meet the requirements of SABS 1200LB for bedding sands.

Notes:

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Assessment of data

The slope stability of the site appears suitable, however if moisture is allowed into any open trenches the stability of the slopes could be affected.

The trench side walls are also stable under dry conditions, however the Ferricrete layer could form a perched water table which could lead to the sided wall failure of the trench. This is evident in TP25 with water seepage in the Ferricrete layer.

The insitu materials are not considered useful as a potential source of bedding, blanket or engineering filling material, due to the high fines content and classification, but if required can be used as general low quality filling material where compaction is not crucial.

Trial pit excavations to a depth of 2.3m are classified as "soft" and can be excavated with light plant such as a TLB. There are also areas along the pipe route with exposed rock outcrops.

Conclusion

The investigation indicated that the site is suitable for the pipeline upgrade with the material being easily excavatable with a TLB in most areas and rock out crops along the line. The site is stable under dry conditions and care must be taken to shore the walls of the trenches where any possible perched water table could be encountered.

Consideration must be taken when selection the backfill material from the site and bedding material should be imported from commercial sources.

Please contact me should you have any queries.

Yours Sincerely

Llewelyn Heathcote B-Tech. (Civil Eng.) & BSc Hons (Transport)
For Outeniqua Lab (Pty) Ltd

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Figure 1:

Scale:NTS

Locality Plan



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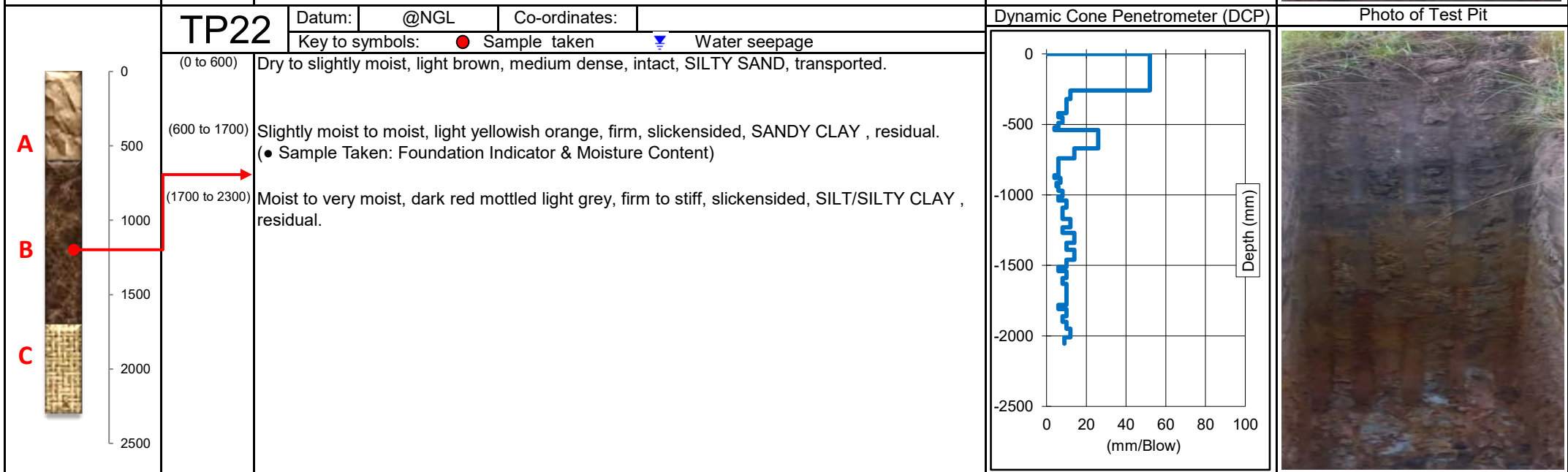
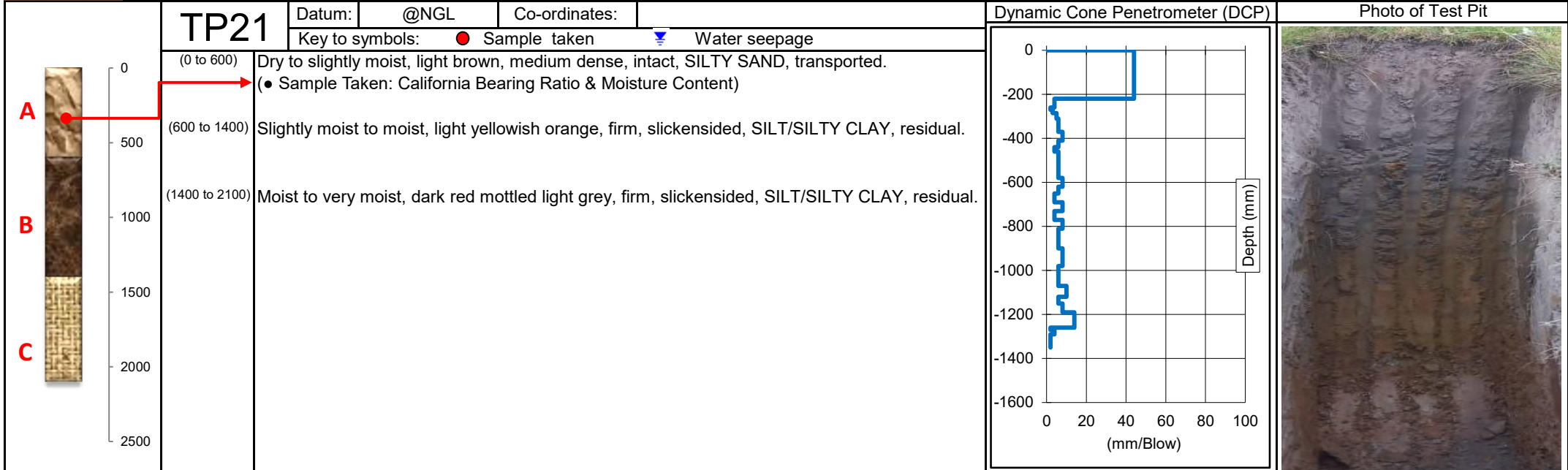
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R-PROF-1-5

Jan-22

Customer :	Neil Lyners P O Box 757 George 6530
Project :	19118CGAC Kwanokuthula Main Outfall Sewer & Pumping Main
Date Reported :	24/04/25



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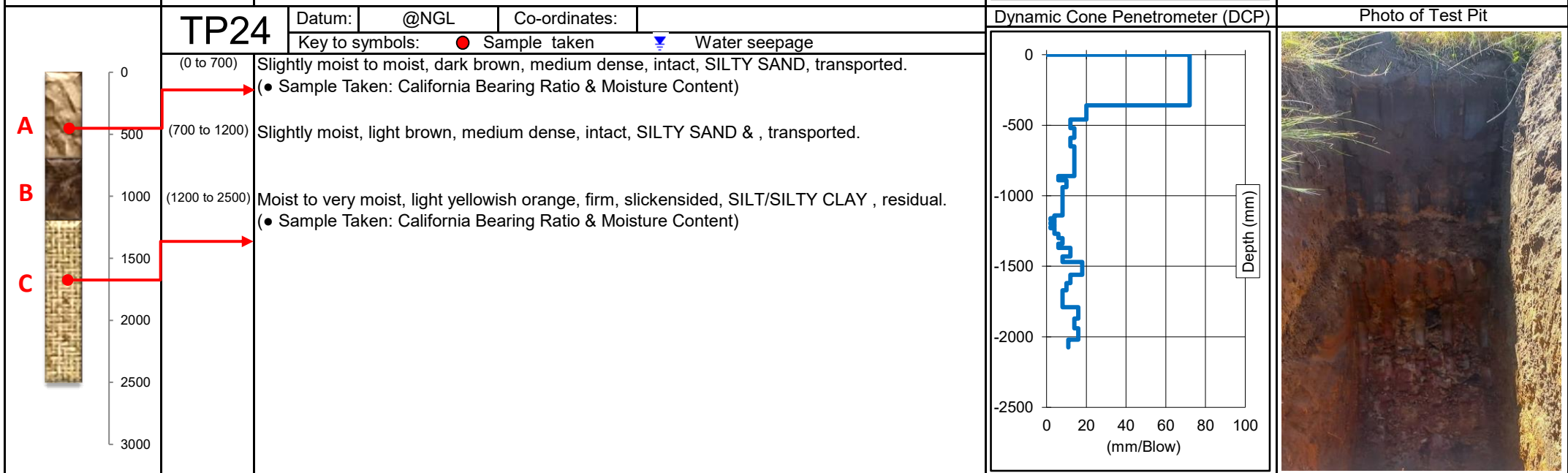
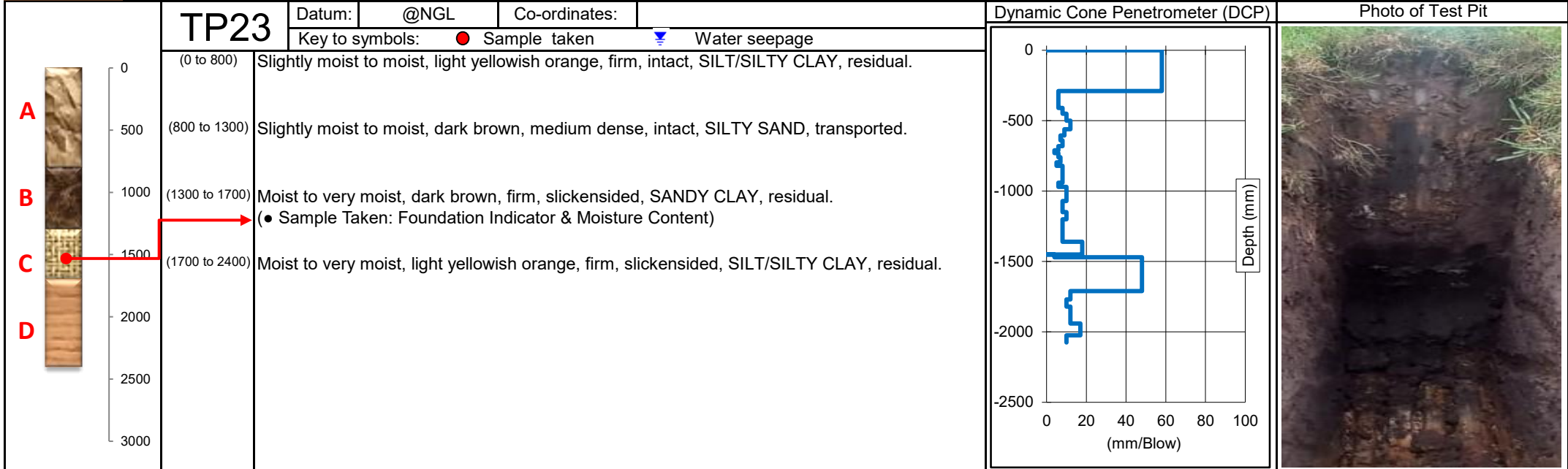
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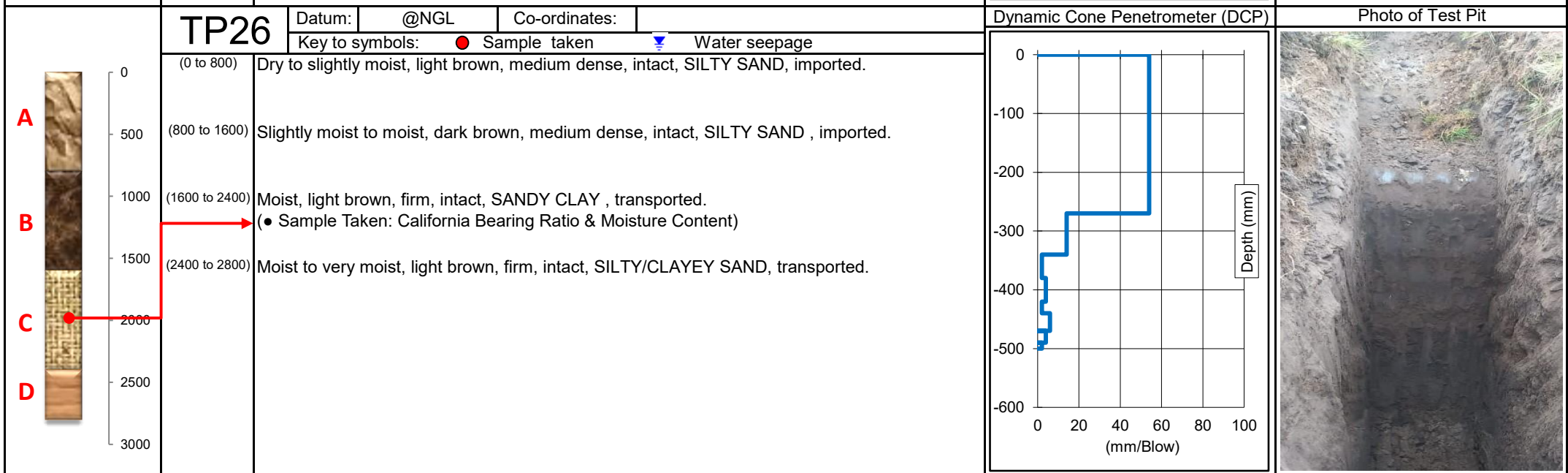
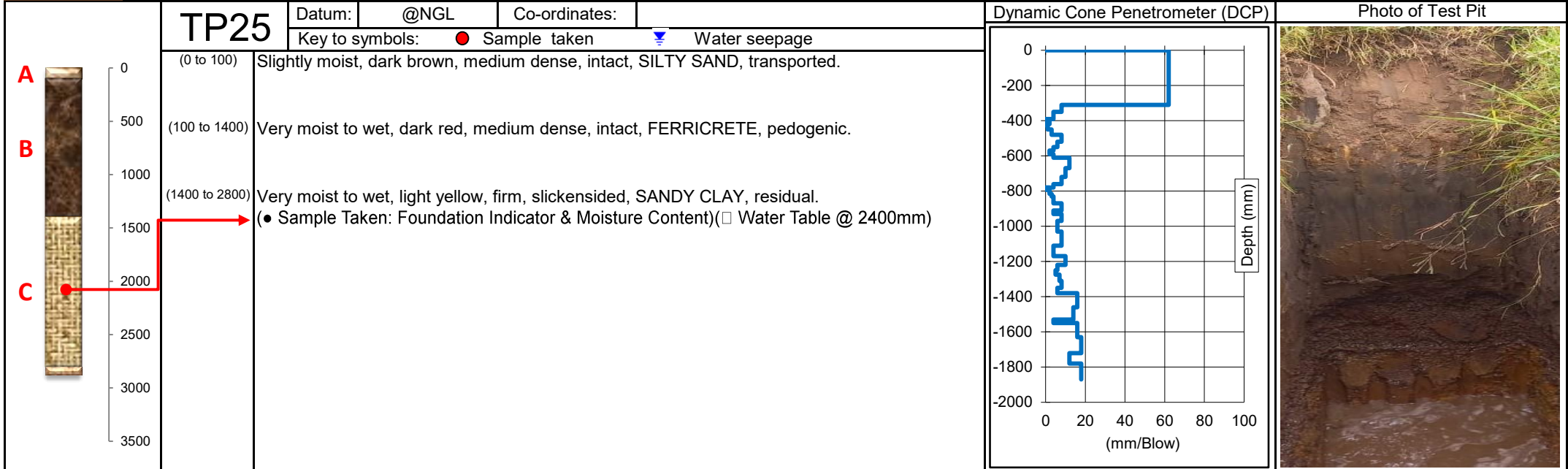
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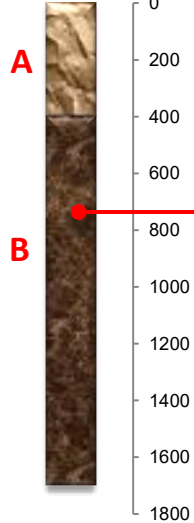
TP27

Datum:	@NGL	Co-ordinates:	
Key to symbols: ● Sample taken ▽ Water seepage			

(0 to 400) Dry to slightly moist, light brown, medium dense, intact, SILTY SAND, transported.

(400 to 1700) Slightly moist to moist, light yellow, medium dense, intact, SILTY GRAVEL WITH BOULDERS, transported.

(● Sample Taken: California Bearing Ratio & Moisture Content)



Dynamic Cone Penetrometer (DCP)

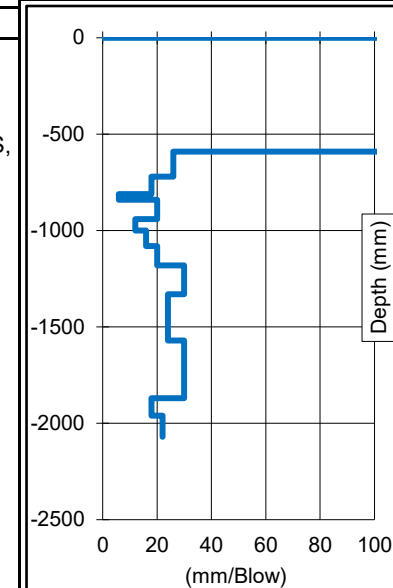


Photo of Test Pit





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R-CBR-1-9

Jun-23



T0347

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		Date Received :	28/03/25
		Date Reported :	24/04/25
		Req. Number :	1829/25
Attention :	Francois van Eck	No. of Pages :	1 of 4

TEST REPORT CALIFORNIA BEARING RATIO

Sample Position (SV)		TP 21	COTO DS:				90876
Depth (mm)		0 - 600	Not				
Sample No		90876	Classified				
Materials Description	Source	In-Situ		Opinion			
	Colour	Light Brown					
	Soil Type	Silty Sand					
	Classification	Trail Pit					
Material Indicators - (SANS 3001 Method GR1)							
Percentage Passing	75 mm	100					
	63 mm	100					
	50 mm	100					
	37.5 mm	100					
	28 mm	100					
	20 mm	100					
	14 mm	100					
	5 mm	99					
	2 mm	99					
	0.425 mm	99					
0.075 mm	66.2						
Material Indicators - (SANS 3001 Method PR5)							
Grading Modulus *		0.36					
Coarse Sand Soil-Mortar (%)		1					
Atterberg Limits - (SANS 3001 Method GR10)							
Liquid Limit (%)		19					
Plasticity Index (%)		4					
Linear Shrinkage (%)		2.0					
Material Strength - (SANS 3001 Method GR30,GR40 - SCALPED)							
MDD	Max Dry Density (kg/m ³)	1945					
	Optimum Moisture Content (%)	9.1					
	Mould Moisture Content (%)	9.0					
A	Relative Compaction (%)	100.0					
	Swell (%)	0.2					
B	Relative Compaction (%)	95.1					
	Swell (%)	0.4					
C	Relative Compaction (%)	91.6					
	Swell (%)	0.6					
CBR	@100% Max Dry Density	34					
	@98% Max Dry Density	22					
	@95% Max Dry Density	12					
	@93% Max Dry Density	7					
	@90% Max Dry Density	4					
Material Condition - (SANS 3001 Method GR20)							
Insitu Moisture Content (%)		5.5					
Soil Classification Of The Material Based Only On The Tests Results Above							
COTO DS Specification:		Not Classified					
AASHTO System		A-4					
Unified System		CL-ML					

Sieve Analysis

CBR Chart

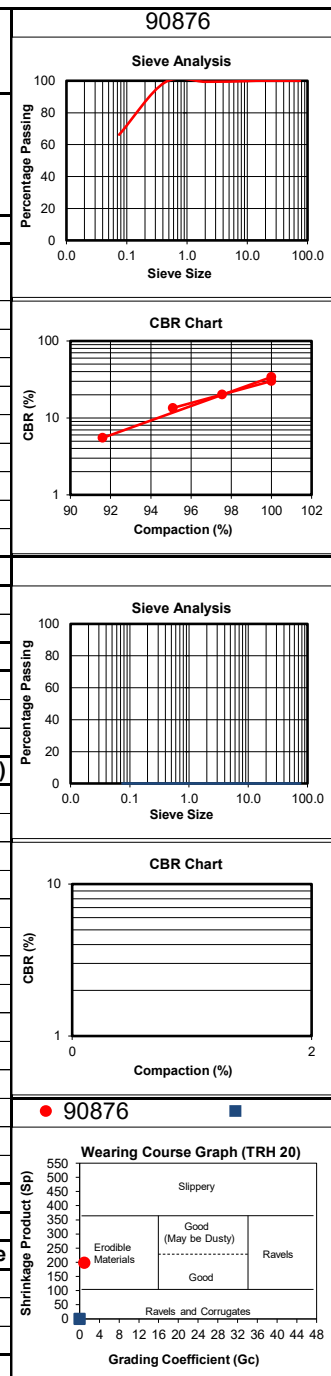
Sieve Analysis

CBR Chart

Wearing Course Graph (TRH 20)

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- Specimens delivered to Outeniqua Lab in good order.



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Ruaan Lesch
Technical Signatory
For Outeniqua Lab (Pty) Ltd.



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R-CBR-1-9

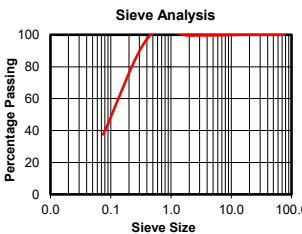
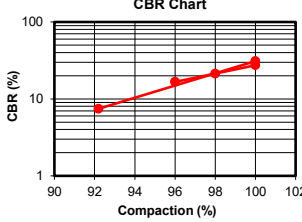
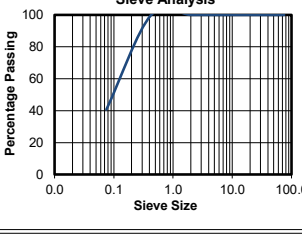
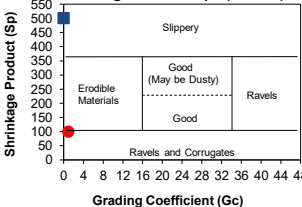
Jun-23



T0347

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Attention :	Francois van Eck	No. of Pages :	2 of 4

TEST REPORT CALIFORNIA BEARING RATIO

Sample Position (SV)		TP 24	COTO DS:		TP 24	COTO DS:		90879	
Depth (mm)		0 - 700	Not		1200 - 2500	No Spec			
Sample No		90879	Classified		90880				
Materials Description	Source	Trial Pit			Trial Pit				
	Colour	Dark Brown			Light Yellowish Orange				
	Soil Type	Silty Sand			Silty /Silty Clay				
	Classification	Insitu			Insitu				
Material Indicators - (SANS 3001 Method GR1)									
Percentage Passing	75 mm	100		Opinion	100				
	63 mm	100			100				
	50 mm	100			100				
	37.5 mm	100			100				
	28 mm	100			100				
	20 mm	100			100				
	14 mm	100			100				
	5 mm	100			100				
	2 mm	99			100				
	0.425 mm	99			100				
0.075 mm	37.6			40.7					
Material Indicators - (SANS 3001 Method PR5)									
Grading Modulus *		0.64			0.59				
Coarse Sand Soil-Mortar (%)		1			0				
Atterberg Limits - (SANS 3001 Method GR10)									
Liquid Limit (%)		15			29				
Plasticity Index (%)		2			10				
Linear Shrinkage (%)		1.0			5.0				
Material Strength - (SANS 3001 Method GR30,GR40 - SCALPED)									
MDD	Max Dry Density (kg/m³)	1967			1894				● 90879 ■ 90880 
	Optimum Moisture Content (%)	8.6			11.9				
		Mould Moisture Content (%)	8.6			11.6			
	A	Relative Compaction (%)	100.0			100.0			
Swell (%)		0.0			2.4				
B	Relative Compaction (%)	96.0			95.5				
	Swell (%)	0.0			2.7				
C	Relative Compaction (%)	92.2			92.9				
	Swell (%)	0.0			2.9				
CBR	@100% Max Dry Density	31			4				
	@98% Max Dry Density	21			3				
	@95% Max Dry Density	12			2				
	@93% Max Dry Density	9			1				
	@90% Max Dry Density	5			1				
Material Condition - (SANS 3001 Method GR20)									
Insitu Moisture Content (%)		7.6			13.9				
Soil Classification Of The Material Based Only On The Tests Results Above									
COTO DS Specification:		Not Classified			Not Classified				
AASHTO System		A-4			A-4				
Unified System		SM			SC				

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- Tests marked with a (*) are NOT SANAS Accredited results.
- Specimens sampled by Outeniqua Lab according to sampling Plan TMH 5 Methods MA2 (Trial Pit).
- Specimens sampled by Boss Sangwe
- The weather conditions were such that there was no detrimental effect on the sample/s taken.

Llewelyn Heathcote

Technical Signatory

For Outeniqua Lab (Pty) Ltd.

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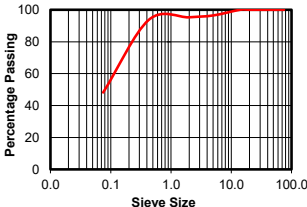
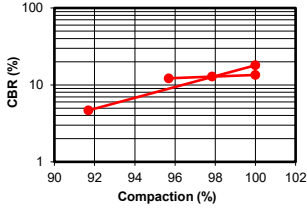
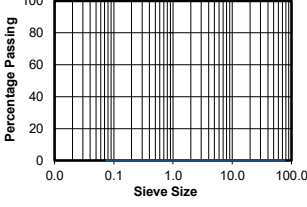
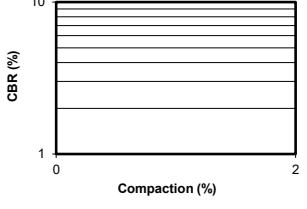
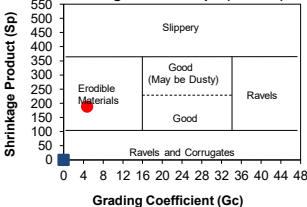
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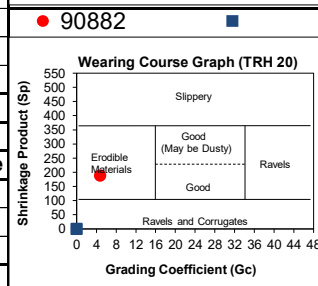
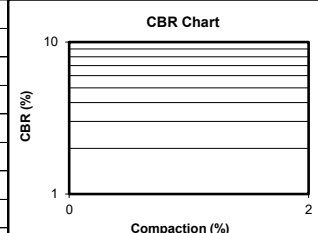
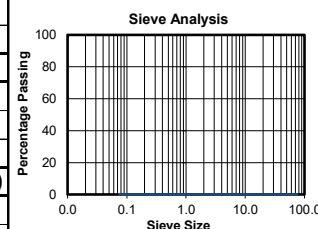
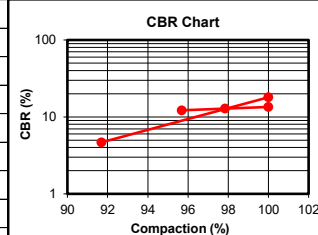
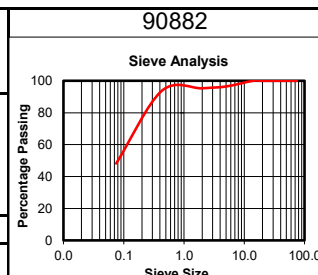


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TEST REPORT CALIFORNIA BEARING RATIO

Sample Position (SV)		TP 26	COTO DS:					90882	
Depth (mm)		1600 - 2400	Not						
Sample No		90882	Classified						
Materials Description	Source	Trial Pit							
	Colour	Light Brown							
	Soil Type	Sandy Clay							
	Classification	Insitu							
Material Indicators - (SANS 3001 Method GR1)									
Percentage Passing	75 mm	100		Opinion					
	63 mm	100							
	50 mm	100							
	37.5 mm	100							
	28 mm	100							
	20 mm	100							
	14 mm	100							
	5 mm	96							
	2 mm	95							
	0.425 mm	94							
0.075 mm	48.2								
Material Indicators - (SANS 3001 Method PR5)									
Grading Modulus *		0.63							
Coarse Sand Soil-Mortar (%)		2							
Atterberg Limits - (SANS 3001 Method GR10)									
Liquid Limit (%)		16							
Plasticity Index (%)		4							
Linear Shrinkage (%)		2.0							
Material Strength - (SANS 3001 Method GR30,GR40 - SCALPED)									
MDD	Max Dry Density (kg/m³)	1988							
	Optimum Moisture Content (%)	10.2							
	Mould Moisture Content (%)	10.2							
A	Relative Compaction (%)	100.0							
	Swell (%)	0.2							
B	Relative Compaction (%)	95.7							
	Swell (%)	0.3							
C	Relative Compaction (%)	91.7							
	Swell (%)	0.5							
CBR	@100% Max Dry Density	18							
	@98% Max Dry Density	13							
	@95% Max Dry Density	8							
	@93% Max Dry Density	6							
	@90% Max Dry Density	4							
Material Condition - (SANS 3001 Method GR20)									
Insitu Moisture Content (%)		10.5							
Soil Classification Of The Material Based Only On The Tests Results Above									
COTO DS Specification:		Not Classified							
AASHTO System		A-4							
Unified System		SM-SC							

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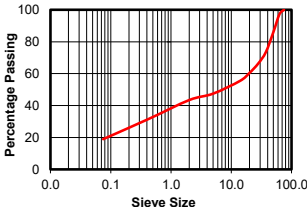


Customer :	Neil Lyners P O Box 757 George 6530	Project :	19118CGAC Kwanokuthula Main Outfall Sewer & Pumping Main
		Date Received :	28/03/25
		Date Reported :	24/04/25
		Req. Number :	1829/25
Attention :	Francois van Eck	No. of Pages :	4 of 4

TEST REPORT CALIFORNIA BEARING RATIO

Sample Position (SV)		TP 27	COTO DS:				90883	
Depth (mm)		400 - 1700	G6 Subbase					
Sample No		90883						
Materials Description	Source	Trial Pit						
	Colour	Light Yellow						
	Soil Type	Sandy Gravel with Boulders						
	Classification	Insitu						
Material Indicators - (SANS 3001 Method GR1)								
Percentage Passing	75 mm	100		Opinion				
	63 mm	97						
	50 mm	86						
	37.5 mm	73						
	28 mm	66						
	20 mm	60						
	14 mm	55						
	5 mm	47						
	2 mm	43						
	0.425 mm	32						
0.075 mm	19.0							
Material Indicators - (SANS 3001 Method PR5)								
Grading Modulus *		2.06	1.50 - 2.50	✓				
Coarse Sand Soil-Mortar (%)		27						
Atterberg Limits - (SANS 3001 Method GR10)								
Liquid Limit (%)		24						
Plasticity Index (%)		7	≤ 14	✓				
Linear Shrinkage (%)		3.5	≤ 7	✓				
Material Strength - (SANS 3001 Method GR30,GR40 - SCALPED)								
MDD	Max Dry Density (kg/m ³)	1934						
	Optimum Moisture Content (%)	11.4						
	Mould Moisture Content (%)	11.3						
A	Relative Compaction (%)	100.0						
	Swell (%)	0.0	≤ 0.5	✓				
B	Relative Compaction (%)	94.5						
	Swell (%)	0.0						
C	Relative Compaction (%)	90.7						
	Swell (%)	0.0						
CBR	@100% Max Dry Density	65						
	@98% Max Dry Density	44						
	@95% Max Dry Density	24	≥ 25	*				
	@93% Max Dry Density	16						
	@90% Max Dry Density	9						
Material Condition - (SANS 3001 Method GR20)								
Insitu Moisture Content (%)		5.6						
Soil Classification Of The Material Based Only On The Tests Results Above								
COTO DS Specification:		G6 Subbase						
AASHTO System		A-2-4						
Unified System		GM-GC						

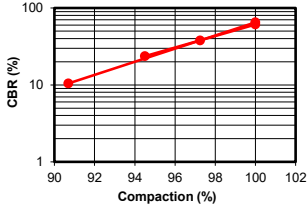
Sieve Analysis



Percentage Passing

Sieve Size

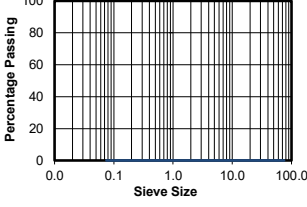
CBR Chart



CBR (%)

Compaction (%)

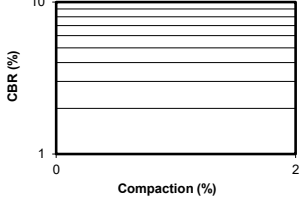
Sieve Analysis



Percentage Passing

Sieve Size

CBR Chart



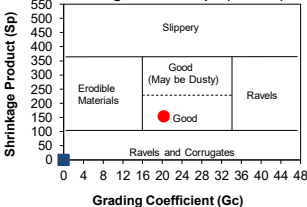
CBR (%)

Compaction (%)

90883



Wearing Course Graph (TRH 20)



Shrinkage Product (Sp)

Grading Coefficient (Gc)

Slippery

Erodible Materials

Good (May be Dusty)

Good

Ravels and Corrugates

Ravels

Tests marked with a (*) are NOT SANAS Accredited results.

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- Specimens sampled by Outeniqua Lab according to sampling Plan TMH 5 Methods MA2 (Trial Pit).
- Specimens sampled by Boss Sangwe
- The weather conditions were such that there was no detrimental effect on the sample/s taken.

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Technical Signatory
For Outeniqua Lab (Pty) Ltd.

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- The opinion column is an interpretation of the direct comparison between the quoted specification and the single test sample results obtained. The compliant (✓), non compliant (✗) and uncertain (*) opinion indicators are based on an approximate 95% level of confidence with reference to SAMM GUIDANCE 1, Issue 2 : 20 June 2007 Section 2.
- The uncertain (*) indicates that the test result is either equal to or is above / below the specified limit by a margin less than the measurement uncertainty; it is therefore not possible to state compliant (✓) or non compliant (✗) based on a 95% level of confidence with reference to SAMM GUIDANCE 1, Issue 2 : 20 June 2007 Section 2.
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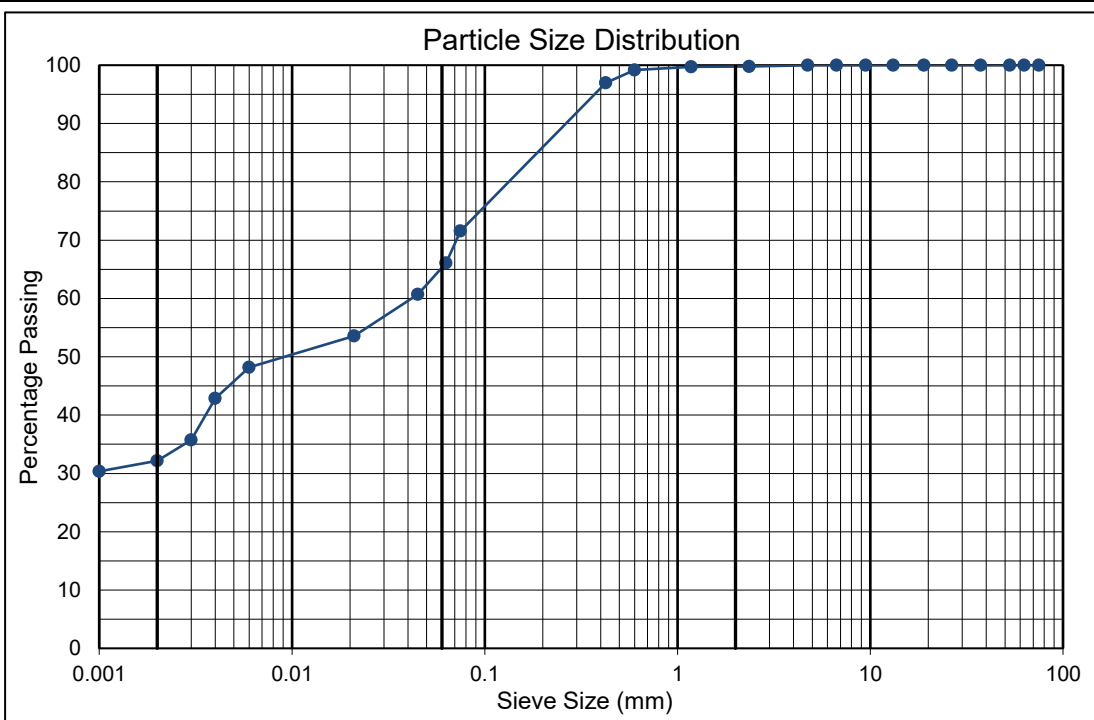
T0347

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Attention :	Francois van Eck	Date Received :	28/03/25
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		No. of Pages :	1 of 3

TEST REPORT FOUNDATION INDICATOR - (ASTM Method D422)

Sample Position (SV)	TP 22
Depth (mm):	600 - 1700
Sample No.:	90877
Materials Description	Trial Pit
Source	Light Yellowish Orange
Colour	Sandy Clay
Soil Type	Insitu
Classification	

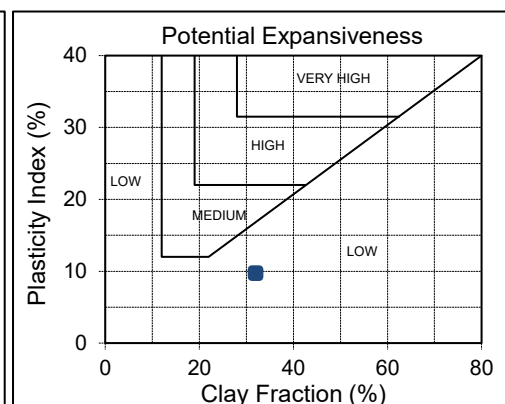
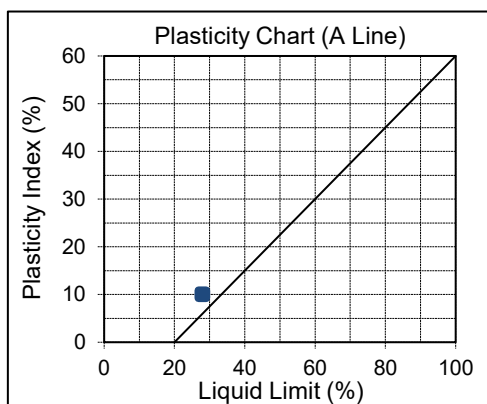
75.0mm	100
63.0mm	100
53.0mm	100
37.5mm	100
26.5mm	100
19mm	100
13.2mm	100
9.5mm	100
6.7mm	100
4.75mm	100
2.36mm	100
1.18mm	100
0.6mm	99
0.425mm	97
0.075mm	72
0.063mm	66
0.045mm	61
0.021mm	54
0.006mm	48
0.004mm	43
0.003mm	36
0.002mm	32
0.001mm	30



Liquid Limit (%)	28
Plasticity Index (%)	10
Linear Shrinkage (%)	5
Moisture Content (%)	20.1

% Clay	32
% Silt	33
% Sand	35
% Gravel	0

Unified Soil Classification	CL
AASHTO Soil Classification	A-4



- Specimen sampled by Outeniqua Lab according to sampling Plan TMH 5 Methods MA2 (Trial Pit).
- Specimens sampled by Boss Sangwe
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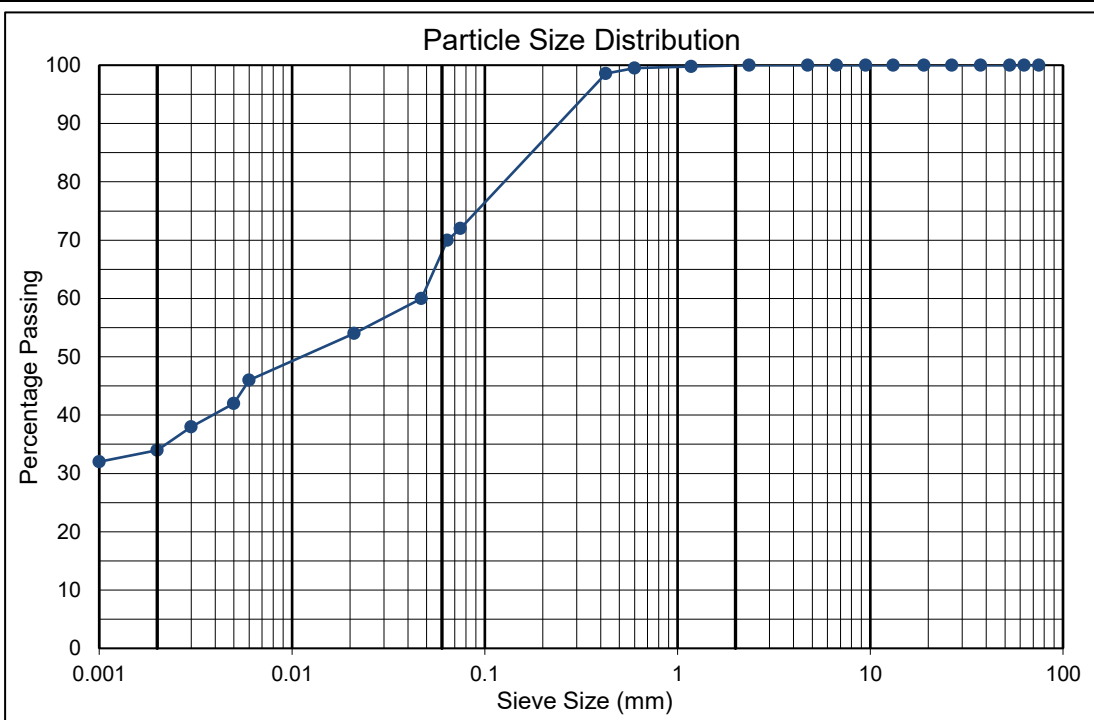
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		No. of Pages :	2 of 3

TEST REPORT FOUNDATION INDICATOR - (ASTM Method D422)

Sample Position (SV)	TP 23
Depth (mm):	1300 - 1700
Sample No.:	90878
Materials Description	Trial Pit Dark Brown Sandy Clay Insitu

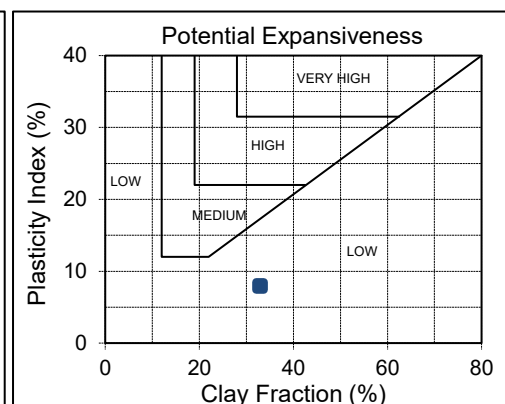
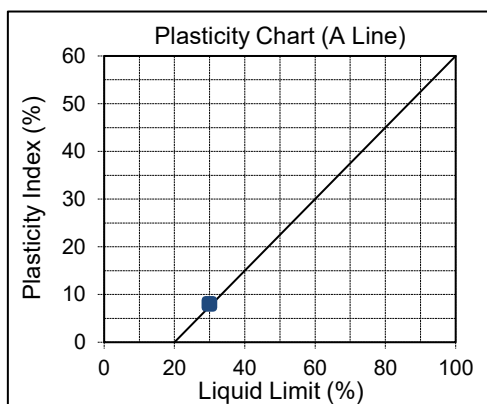
75.0mm	100
63.0mm	100
53.0mm	100
37.5mm	100
26.5mm	100
19mm	100
13.2mm	100
9.5mm	100
6.7mm	100
4.75mm	100
2.36mm	100
1.18mm	100
0.6mm	100
0.425mm	99
0.075mm	72
0.064mm	70
0.047mm	60
0.021mm	54
0.006mm	46
0.005mm	42
0.003mm	38
0.002mm	34
0.001mm	32



Liquid Limit (%)	30
Plasticity Index (%)	8
Linear Shrinkage (%)	4
Moisture Content (%)	22.0

% Clay	33
% Silt	35
% Sand	32
% Gravel	0

Unified Soil Classification	CL
AASHTO Soil Classification	A-4



- Specimen sampled by Outeniqua Lab according to sampling Plan TMH 5 Methods MA2 (Trial Pit).
- Specimens sampled by Boss Sangwe
- The weather conditions were such that there was no detrimental effect on the sample/s taken.

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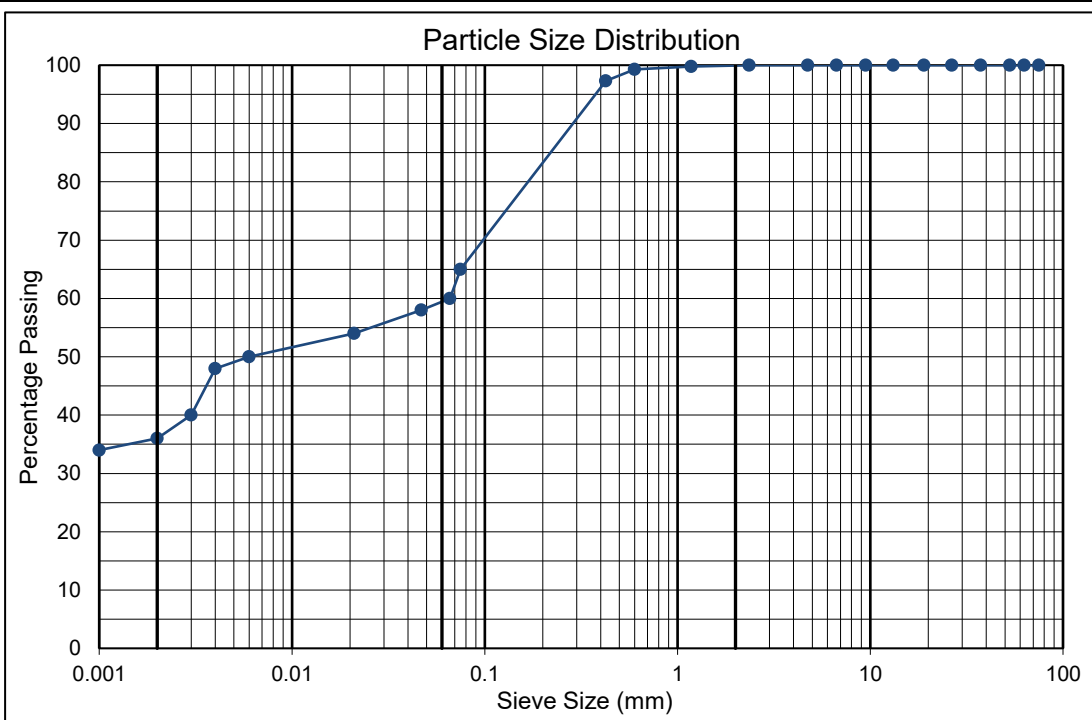
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		No. of Pages :	3 of 3

TEST REPORT FOUNDATION INDICATOR - (ASTM Method D422)

Sample Position (SV)	TP 25
Depth (mm):	1400 - 2800
Sample No.:	90881
Materials Description	Trial Pit Light Yellow Sandy Clay Insitu

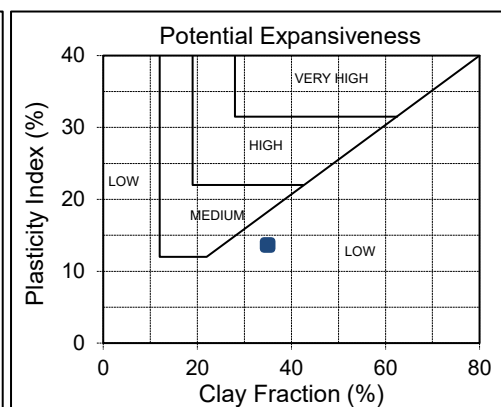
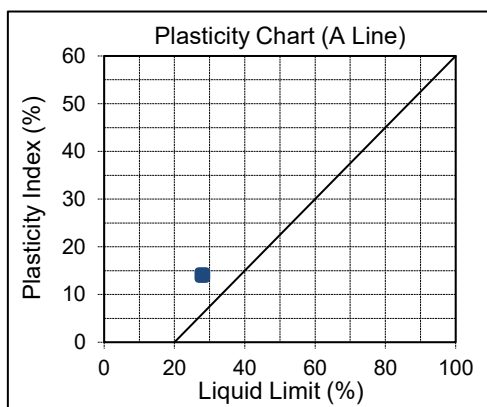
75.0mm	100
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53.0mm	100
37.5mm	100
26.5mm	100
19mm	100
13.2mm	100
9.5mm	100
6.7mm	100
4.75mm	100
2.36mm	100
1.18mm	100
0.6mm	99
0.425mm	97
0.075mm	65
0.066mm	60
0.047mm	58
0.021mm	54
0.006mm	50
0.004mm	48
0.003mm	40
0.002mm	36
0.001mm	34



Liquid Limit (%)	28
Plasticity Index (%)	14
Linear Shrinkage (%)	7
Moisture Content (%)	26.8

% Clay	35
% Silt	24
% Sand	41
% Gravel	0

Unified Soil Classification	CL
AASHTO Soil Classification	A-6



- Specimen sampled by Outeniqua Lab according to sampling Plan TMH 5 Methods MA2 (Trial Pit).
- Specimens sampled by Boss Sangwe
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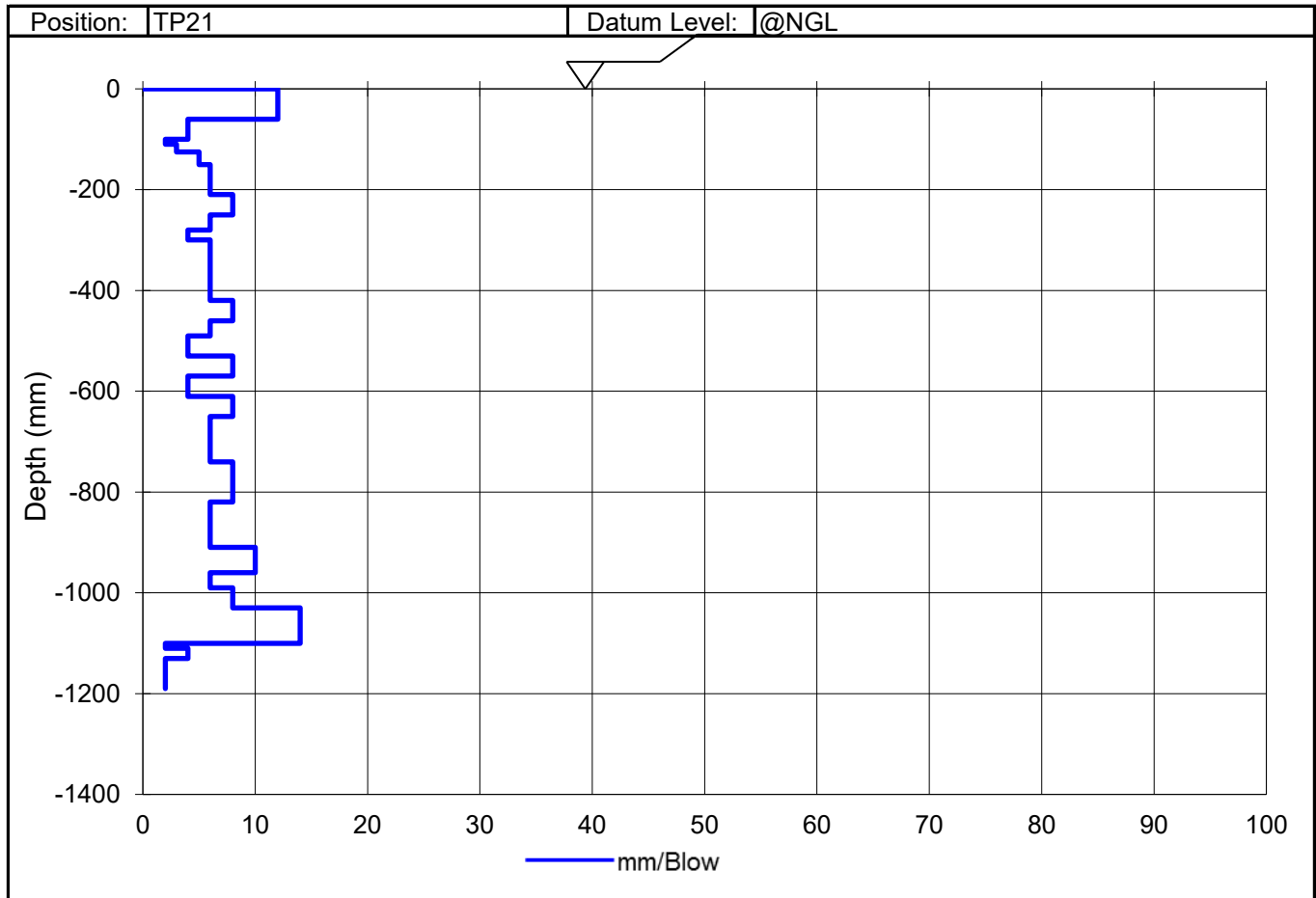


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TEST REPORT

DYNAMIC CONE PENETROMETER (DCP) - (SANS 3001 - Method Pending)



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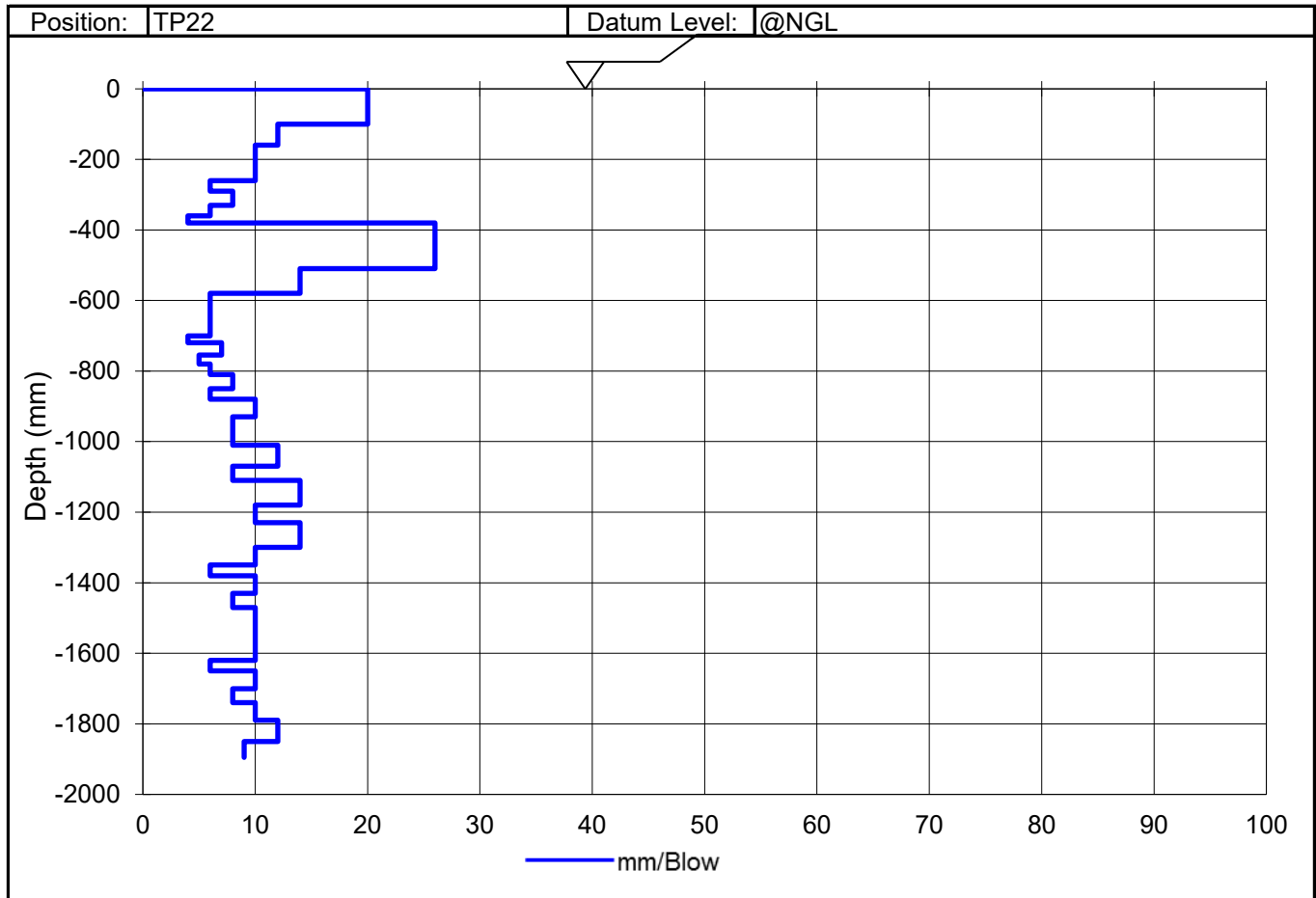


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TEST REPORT

DYNAMIC CONE PENETROMETER (DCP) - (SANS 3001 - Method Pending)



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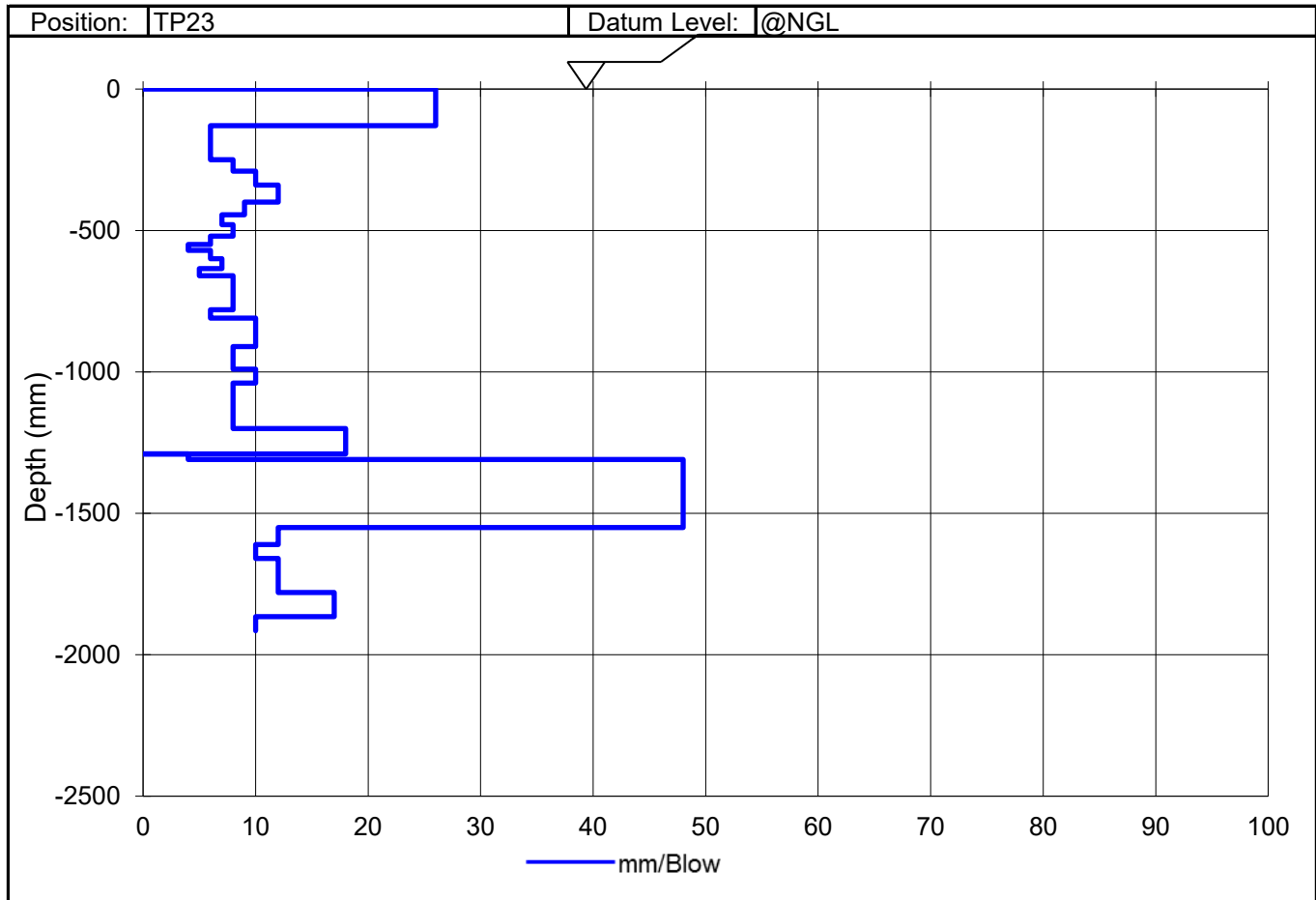


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DYNAMIC CONE PENETROMETER (DCP) - (SANS 3001 - Method Pending)



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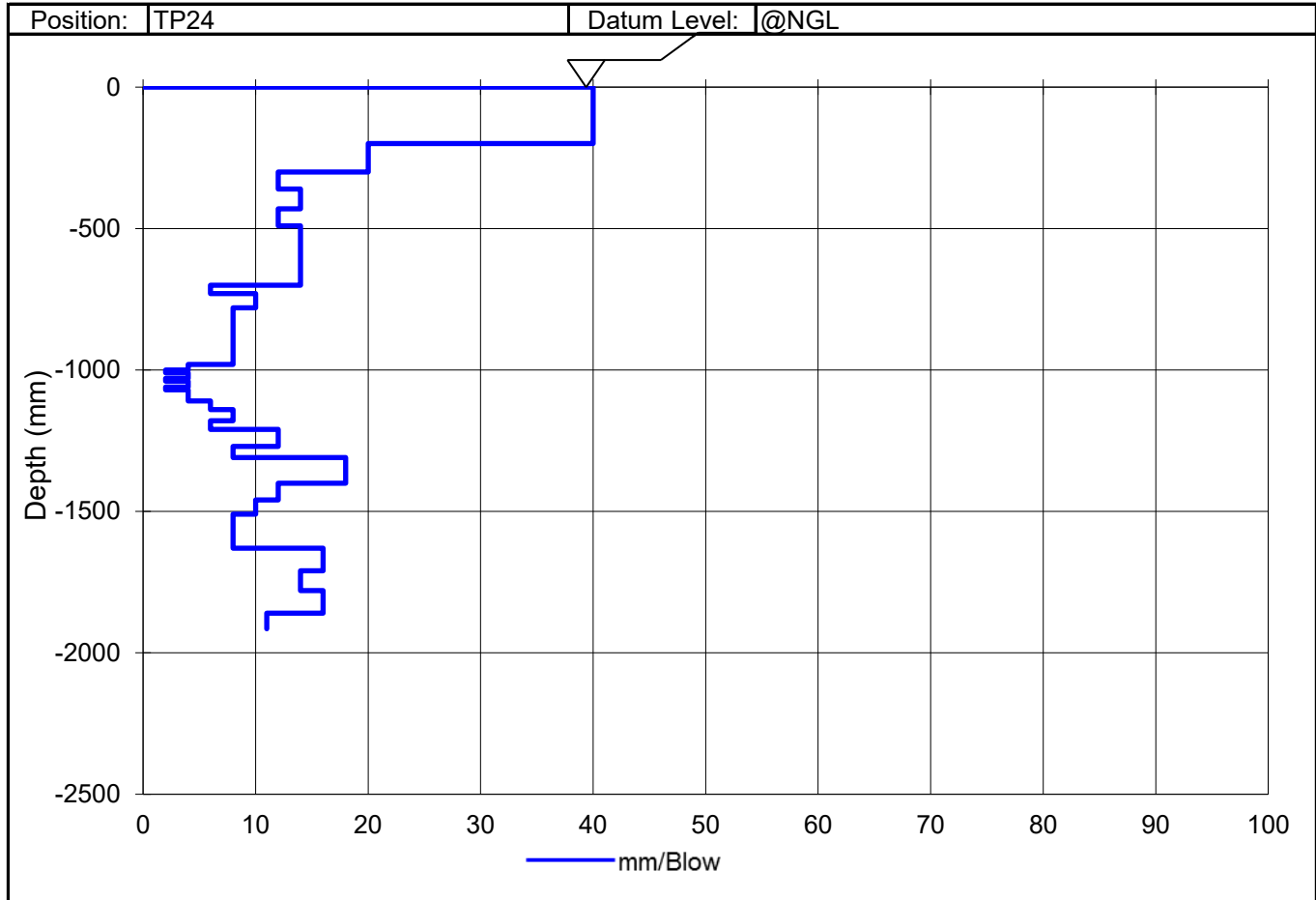


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DYNAMIC CONE PENETROMETER (DCP) - (SANS 3001 - Method Pending)



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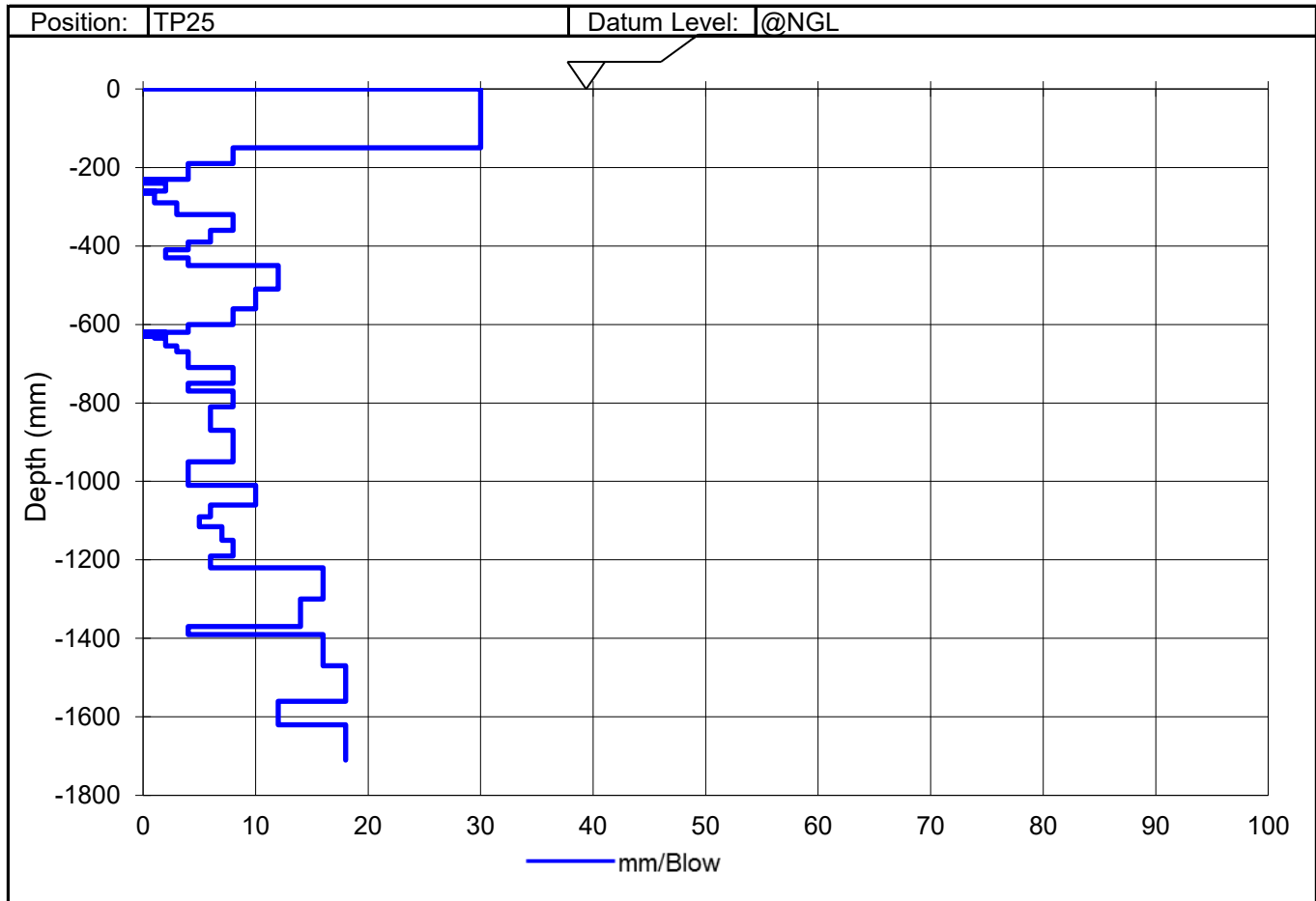


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TEST REPORT

DYNAMIC CONE PENETROMETER (DCP) - (SANS 3001 - Method Pending)



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Jun-21

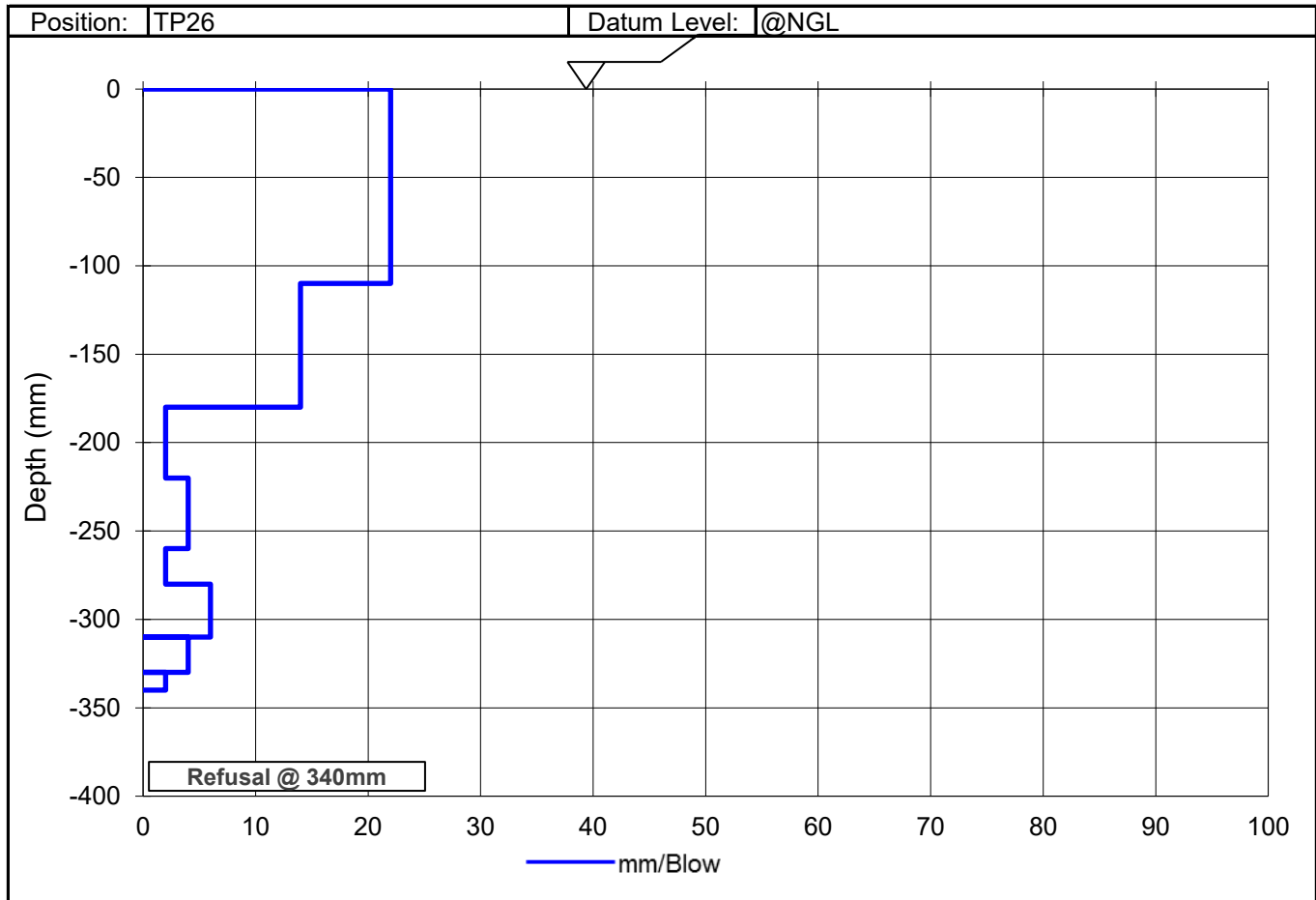


T0347

Customer :	Neil Lyners P O Box 757 George 6530	Project :	19118CGAC Kwanokuthula Main Outfall Sewer & Pumping Main
		Date Received :	28/03/25
		Date Reported :	24/04/25
		Req. Number :	1829/25
Attention :	Francois van Eck	No. of Pages :	6 of 7

TEST REPORT

DYNAMIC CONE PENETROMETER (DCP) - (SANS 3001 - Method Pending)



Notes:

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Llewelyn Heathcote
Technical Signatory
For Outeniqua Lab (Pty) Ltd.

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OUTENIQUA LAB (Pty) Ltd.

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Jun-21

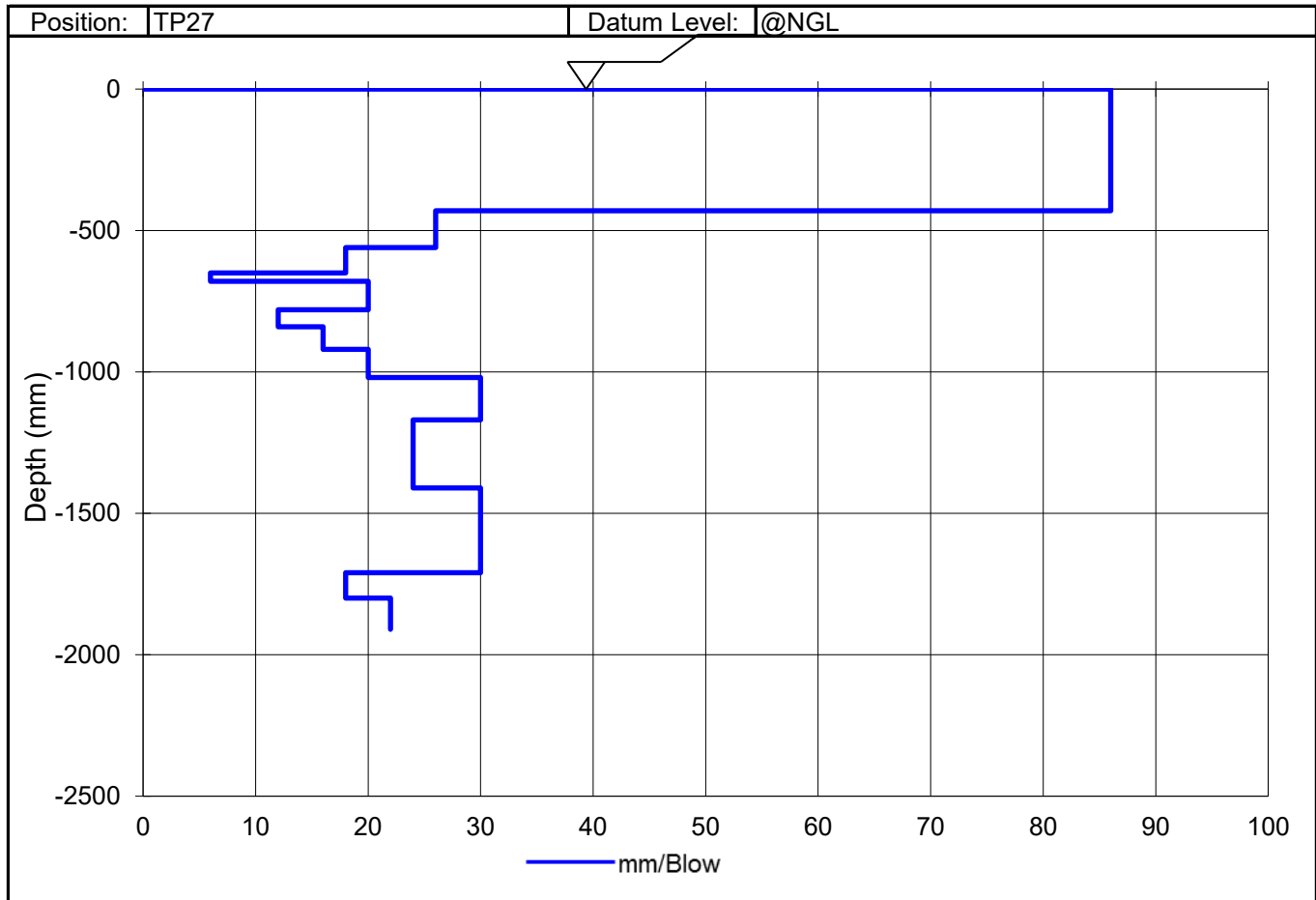


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